

**Free Surface Flow**  
**Dr. Mohammad Saud Afzal**  
**Department of Civil Engineering**  
**Indian Institute of Technology Kharagpur**

**Lecture 33**

Welcome students to yet another session of problem solving in gradually varied flow. So, last class we solved a problem where we were asked to calculate the surface profile for a trapezoidal channel and in that we use different tables and we were able to draw the surface profiles and we were able to see the change in grade as well. So, starting with a new problem now. So now, what we have? We have a rectangular channel.

Earlier with the trapezoidal channel, M and N are in series with reach M being upstream of reach N. These, so here we have only two reach and the difference is it is a rectangular channel. These channels, so these channel reaches have the following characteristics. width, discharge, slope and Manning's number and the reach are M and N. Width is 5 meter in the first reach, in the second reach is 4 meter. Discharge is same 15.0, 15.0 and slope is 0.0004, this is 0.0003 and n is 0.025 and this is 0.015. the question is sketch the resulting gradually varied profile due to change in the channel characteristics as above.

So, this is the question. So, two reaches are given, M and N, with different parameters: width 5, 4; discharge is the same; slopes are different; and Manning's number is also different. So, what we have to do is sketch the resulting gradually varied flow profiles. So, this is a rectangular channel. So, this might not be that difficult to calculate different  $y_c$  and  $y_0$  and other things.

So, we know capital  $Q$ , right? So, right, and therefore, we can calculate small  $q_m$ . In reach M, this demonstrates the reach, and that is  $Q/B$ . So,  $Q_m/B_m$  or 15/5, that is 3 meter cube per second per meter, and similarly, if we have  $q_m$ , then  $y_{cm}$ , that is the critical depth in this particular stretch, is going to be  $\left(\frac{q^2}{2g}\right)^{1/3}$ , that is  $\left(\frac{3^2}{2 \times 9.81}\right)^{1/3}$ . And that gives us 0.9717 meter.

Similarly, for stretch N, so  $Q_n/B_n$ , that is 15. Discharge was the same, but the width is different. So, it is 3.75 meter cube per second per meter, which means  $y_{cn}$  is going to be

$$\left(\frac{q^2}{2g}\right)^{1/3} \text{ or } \left(\frac{3.75^2}{2 \times 9.81}\right)^{1/3} . \text{ or } 1.1275 \text{ meter. So, } y_{cm} \text{ is equal to } 0.9717 \text{ and } y_{cn}.$$

$y_{cm}$  and  $y_{cn}$  is 1.1275 meters. This we have found out very easily since it was a rectangular channel. So, now the important part is the normal depth calculation. This is quite important. So, for normal depth calculation, we start with one reach.

So, let us say reach M here.  $B$  is given as 5 meters,  $Q$  is given as 15 meter cube per second, So is given as 0.0004, and  $n$  is given as 0.025. For a rectangular channel,  $m$  can be given as 0. So, we do almost the same procedure:  $\phi$  is equal to  $\frac{nQ}{\sqrt{S_0 B^{8/3}}}$ , that is  $\frac{0.025 \times 15}{\sqrt{0.0004 \times 5^{8/3}}}$ , and that will give us 0.25665.

For  $\phi$  is equal to 0.2565 and  $m$  is equal to 0, we again look at Table 1. So, we go and start looking at Table 1, and the value we have to look for is 0.2565. 0.25, so 0.2565. So, you see one value is here,  $m$  is equal to 0, and the other value is here, 0.2565. So, interpolation between these two values.

Will result for  $m$  is equal to 0. See, we will see this value; I have already written it down. At Table 1, we get  $\epsilon_0$  is equal to  $y_o/B$  is equal to 0.6071. It was between 0.6 and 0.610.  $y_o$  is equal to  $B \times 0.6071$ , and  $B$  was  $5 \times 0.6071$ .

So,  $y_o$  comes out to be 3.036 meters. And similarly, for reach N, we calculate  $\phi$  is equal to  $\frac{nQ}{\sqrt{S_0 B^{8/3}}}$ . And this is equal to 0.015 and 15 divided by under root 0.0003 and into 4 raised to the power 8 by 3, and this value is going to come to 0.3222. We know that for this particular one,  $B$  was 4 meters.  $Q$  was 15 meters cube per second, So was 0.0003, and  $n$  was 0.015. So, if we look at this table,

Again, for 3222 for  $m$  is equal to 0. So, the value was 0.322.  $M$  is equal to 0, 3, 2, 2, so somewhere between here, and it should come out to be between here. So, it is around 0.725 or something. That for  $\phi$  is equal to 0.322 and  $m$  is equal to 0,  $\eta_o$  is equal to  $y_o/B$  is equal to 0.7254.

That means  $y_0$  is equal to 2.902 meters. Now, we will draw the table, and the table will be filled out. Reach,  $y_0$ ,  $y_c$ , slope, classification. Nature of break in grade and nature of gradually varied flow profile due to break in grade. Now, there are two reaches, M and N.  $y_0$  came out to be 3.3036 in the first case, and here it came out to be 2.902.

Critical was 0.9717, and this was 1.1275. So, you see  $y_0$  is greater than  $y_c$ . So, this  $y_0$  is greater than this  $y_c$ . So, that means it is a mild slope. Here also,  $y_0$  is still greater than  $y_c$ .

This one is greater than this one. So, this is again a mild slope. So, the nature of the break from this section to this section is mild to steeper mild. Here, it is an **M2** curve in reach, and here it is M. So, what happens is the channel slope

changes from mild slope to steeper mild slope and an **M2** curve is formed in the reach M. The curve has the upstream asymptote of  $y_0$  is equal to 3.3036 meter. And this thing we will get it clarified when we draw the figure and ends at the depth of 2.902 meter at the junction of the two reaches.

The figure is below. that is below. This is channel M, this is channel N. NDL NDL and this is **M2** curve. this is CDL, this is CDL 0.9717.

Of course, this is not according to the scale 3.306 meter and here it is 1.1275 and this is this is not up to the scale. So, this is the rough figure, how it will look like. So, we will solve yet another problem. A 2.0 m, wide rectangular channel that is  $n$  is equal to 0.015 carry the discharge of 4 meter cube per second. the channel is laid on a slope of 0.0162.

A downstream sluice gate raises the water surface to 7 meter immediately behind it. question is find the transitional depth. So, the question is there is a wide 2 meter wide rectangular channel that carries a discharge of 4 meter cube per second. The channel is laid on a slope of 0.0162.

A downstream sluice gate raises the water surface to 7 meter immediately behind it. Find the transitional depth. So, in the theory, what we have seen is from uniform flow topic for critical slope in case of wide rectangular channel the limit value of  $S_{*c}$  is equal to  $\frac{S_{Lc} B^{1/3}}{gn^2}$

and that is equal to 2.667 and the limit slope  $S_{LE}$  sorry  $S_{Lc}$   $\frac{2.667 \times 9.81 \times 0.015^2}{2^{1/3}}$  is equal to

0.004672 since the actual slope  $S_0$  is greater than  $S_{Lc}$ . This is the thing to check if transitional depth is possible or not. So, since  $S_0$  is greater than  $S_{Lc}$  that is the limit slope, transitional depth is possible.

The normal depth  $y_0$  for given  $S_0$ ,  $n$ ,  $B$ , and  $Q$  is found using Table 1, which we have been using in our PPT a lot. So,  $\phi$  is  $\frac{4 \times 0.15}{\sqrt{0.0162 \times 28^{8/3}}}$ . is equal to 0.07424. And if we look for  $\phi$  0.07424 for  $m$  is equal to 0, that is a rectangular channel. I have already looked at it, and you can see that  $y_0/B$  is equal to 0.2466.  $y_0/B$  is equal to 0.2466, which implies  $0.2466 \times B$ ,  $0.2466 \times 2$ , that is 0.493 meters. So, the normal depth here is 0.493 m. Similarly, we shall also find the critical depth, and the critical depth formula  $y_c$  is  $\left(\frac{16}{4 \times 9.81}\right)^{1/3}$  by 0.742 meters.  $y_0$  is less than  $y_c$  is less than  $y$ , the channel is a steep slope channel, and the GVF profile is an S1 curve. So, at the transitional depth that is  $S_{*0c}$  is  $\frac{S_0 B^{1/3}}{gn^2}$ , which is  $\frac{0.0162 \times 2^{1/3}}{9.81 \times 0.0015^2}$ . And that is 9.247, and that is equal to  $\frac{(1+2\eta_t)^{4/3}}{\eta_t^{1/3}}$  So, how do we find that? How do we solve this?

By using trial and error.

The error method: the two depths are found as  $\eta_{t1}$  is equal to 2.985, or  $y_{t1}$  is 5.970, and  $\eta_{t2}$  is equal to 0.00125, or  $y_{t2}$  is equal to 0.0025, both in meters, OK. So, yeah. So, the second transitional depth,  $y_{t2}$ , is not of any significance in this problem.

So, the *SI* curve starting after a jump from the normal depth will continue to rise till  $y$  is equal to  $y_t$ , which is equal to 5.970 meters, at which point it will become horizontal. Beyond  $y$ , the Froude number at  $y$  is equal to  $F_t$  is  $\frac{4/(2 \times 5.97)}{\sqrt{9.81 \times 5.97}}$ , that is 0.0438.

All right, so we were asked to find the transitional depth, which we found out, and this is the answer. It was quite a complicated and long question. Complicated in the sense that it was lengthy; we needed to solve for many things, and we needed to use the tables for the normal depth. The critical depth was simple because it was rectangular. In the end, this is how you are going to approach this type of problem. So, I will end this lecture now, and I will see you in the next class again.

Until then, goodbye.